Quantification of the vulnerability of buildings exposed to the risk of debris flows and flash floods through numerical modelling (Quorum Project CAP2025 Region)

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Itasca symposium



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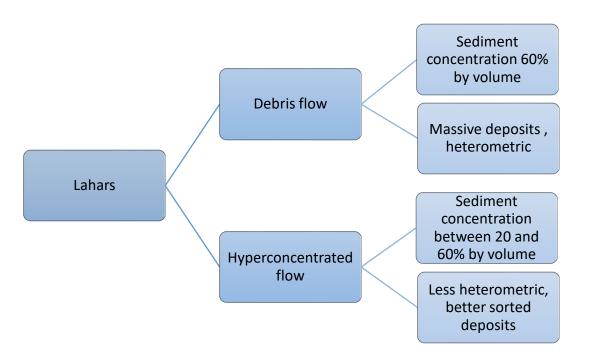
Plan

- 1. Background
- 2. Numerical approach
- 3. Coupling *PFC* with fluid?
- 4. Parametric study
- 5. Comparison with empirical methods
- 6. Conclusions and outlooks

1. Background

• As a result of global warming, mudslides and flash floods (lahars) are increasingly frequent in the latitudes and expose urbanized areas to a significant risk.

Lahar = mixture of sediments and water originating from volcanoes.





damage caused by a mudslide in Mocoa, Colombia, on 3 April 2017.

1. Background

Case study: Arequipa-Peru

- **Location:** 17 km of the summit of the El Misti volcano
- **Hazard:** high precipitation + volcanic ash deposits
- **Consequences:** exposed residential areas, poor populations; infra. exposed

Very high risk for the safety of the population

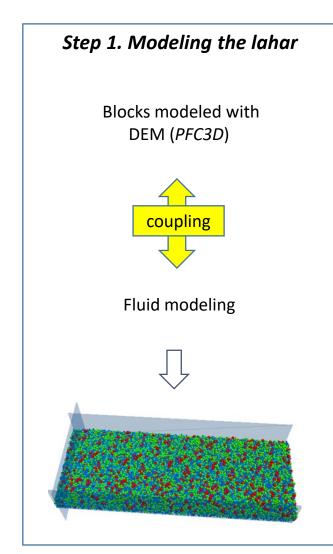
→ It is necessary to assess and map the risk through vulnerability quantification





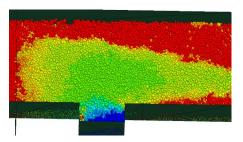
Lahars in Arequipa

General approach

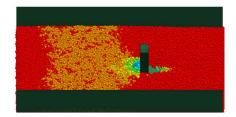


Step 2. Modeling the interaction between lahars and structures

Pressure induced by the flow on different types of obstacles



Lahars in a canal with a recess



Lahars in a canal with an obstacle

Step 3. Assessing the vulnerability of structures

Flow vs. Structure



Stress levels calculated from the total flow pressure



Deformation/damage to quantify the vulnerability of the structure

Modeling scales

✓ Models to obtain trajectory of the flow and global flow characteristics

Existing models

Experimental

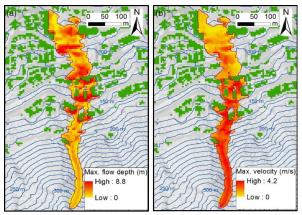


USGS debris-flow flume(Iverson 2010)

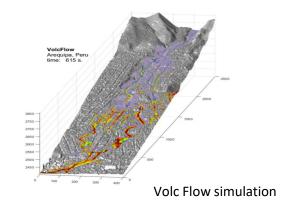


Bugnion et al .2012

Numerical



Gao et al. 2018 (2D flow simulation)

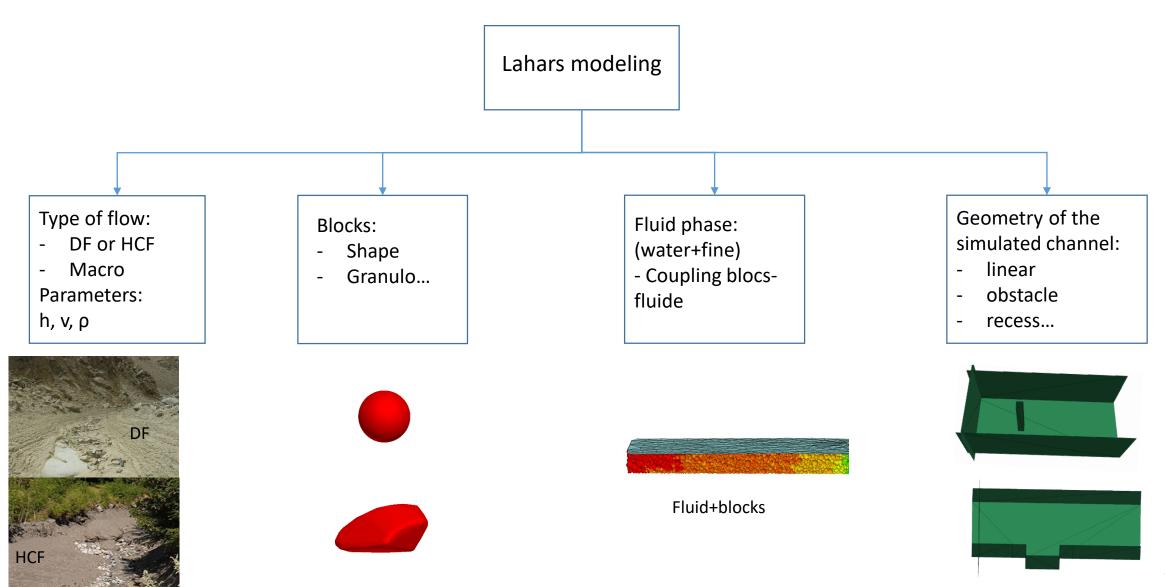


Titan 2F

✓ Modeling at the scale of the structure

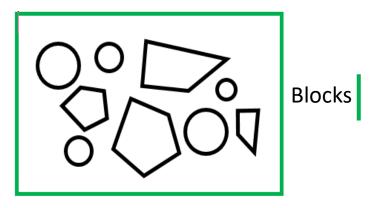
Our numerical model

- Complement existing models at smaller scale
- Export suitable flow parameters to this area (h, v, p)
- Calculate the impact of the blocks on the structures in this region

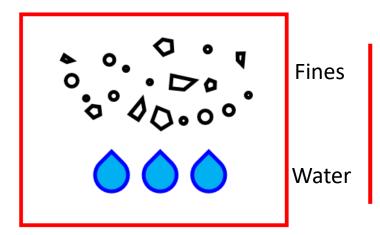


Problem modeling

Solid fraction

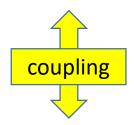


Fluid phase



Modeled explicitly with DEM

✓ Blocks with a given size distribution

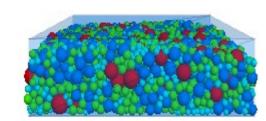


Modeling of the fluid and its effects on blocks

- ✓ Buoyancy effect
- ✓ Drag force

Model procedure

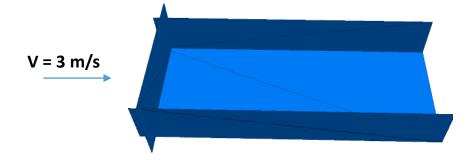
1. Representative Elementary Volume (REV) (5 m \times 10 m \times 1.5 m)



2. Generation of a rectangular channel (25 m \times 10 m \times 4 m)

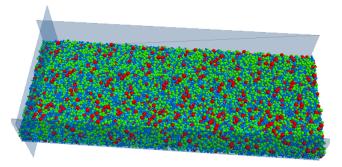
Fluid velocity assumed to be constant and equal to 3 m/s

→ fixed velocity for the blocks at the entrance of the channel then released



Channel flow generation





Model calibration

Blocks density (kg/m³)	Fluid density (kg/m³)	Dynamic viscosity (Pa.s)	Friction ball-ball	Friction Ball-wall	Rolling resistance
Between	Between	Between	Between	Between	Between
2500 et 2700	1000 et 2000	0.03 et 0.075	0.05 et 0.4	0 et 1	0 et 0.6

Iverson (1997)

Solid Grain	Properties	
Mass density, kg/m ³	-	2500-3000
Mean diameter, m	$ ho_s$ δ	$10^{-5} - 10$
Friction angle, deg	$\Phi_{m{g}}$	25-45
Restitution coefficient	e	0.1-0.5
Pore Fluid	Properties	
Mass density, kg/m ³	ρ_f	1000-1200
Viscosity, Pa s	μ	0.001 – 0.1
Mixture P	roperties	
Solid volume fraction	$v_{\rm s}$	0.4 – 0.8
Fluid volume fraction	v_f	0.2 - 0.6
Hydraulic permeability, m ²	$egin{array}{c} oldsymbol{v}_f \ oldsymbol{k} \end{array}$	10^{-13} – 10^{-9}
Hydraulic conductivity, m/s	\boldsymbol{K}	$10^{-7} - 10^{-2}$
Compressive stiffness, Pa	$oldsymbol{E}$	$10^3 - 10^5$
Friction angle, deg	ф	25–45

DF characteristics searched

Solid concentration = 50 % Apparent density = 1800 kg/m³ Flow rate = 35-40 m³/s



Property calibration

Blocks density = 2500 kg/m³
Fluid density = 1100 kg/m³
Dynamic viscosity = 0.048 Pa.s
Friction Ball-Ball = 0.4
Friction wall-ball = 0.5
Rolling resistance = 0.2

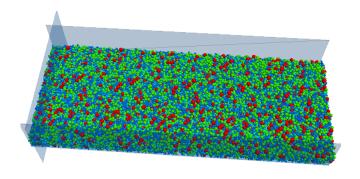


Result

Solid concentration 55 % Apparent density 1867 kg/m³ Flow rate 38-40 m³/s

Geometry of the simulated flow

Straight rectangular channel





Constant velocity
Homogeneous and constant flow field

Complex channel



Choosing a code to model the fluid phase

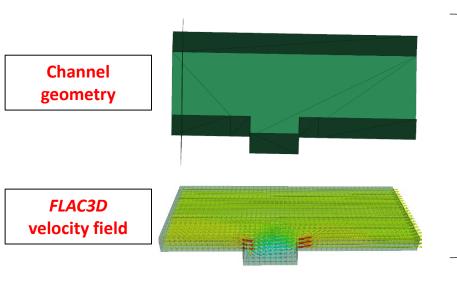


Solution 1: Darcy (with FLAC3D)



Solution 2: *Navier stokes FE (Telemac3D)*

• Solution 1 : coupling with a *Darcy flow*



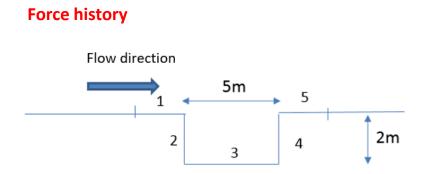
Using FLAC3D flow model (incompressible flow in a porous media)

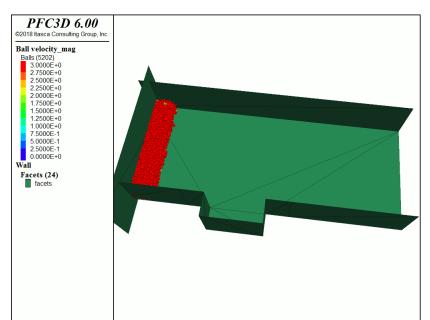


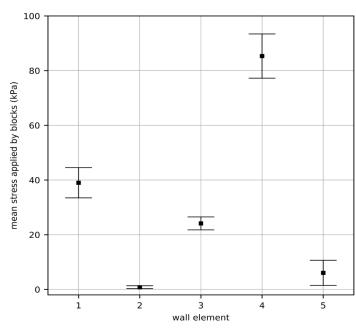
Obtaining the norm and orientation of the fluid velocity vectors in each cell of the FLAC3D mesh



Applying the velocity fields to the particles located in each cell (drag)







• Why Darcy?

- ✓ Easy to couple FLAC3D with PFC3D
- ✓ Two ways coupling
- Access to fluid velocity vectors in each cell of the channel

Darcy Limitations

- ✓ Difficult to model the free surface
- ✓ No turbulences
- ✓ Difficulties to obtain realistic velocity vectors



Solution 2 : Simulate fluid calculation with Navier stokes FE (*Telemac3D*)

Objectif

- ✓ Obtaining a velocity field of a free surface flow with defined boundary conditions
- ✓ Simple turbulence model
- Method for resolving velocity vectors and fluid depth

Limitations

- ✓ Simulate newtonian fluid (water)
- ✓ One way coupling

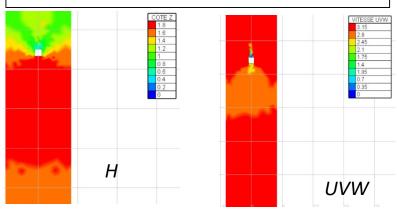
Solution 2 : Telemac3D

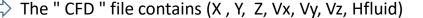
Fluid calculation Telemac 3D:

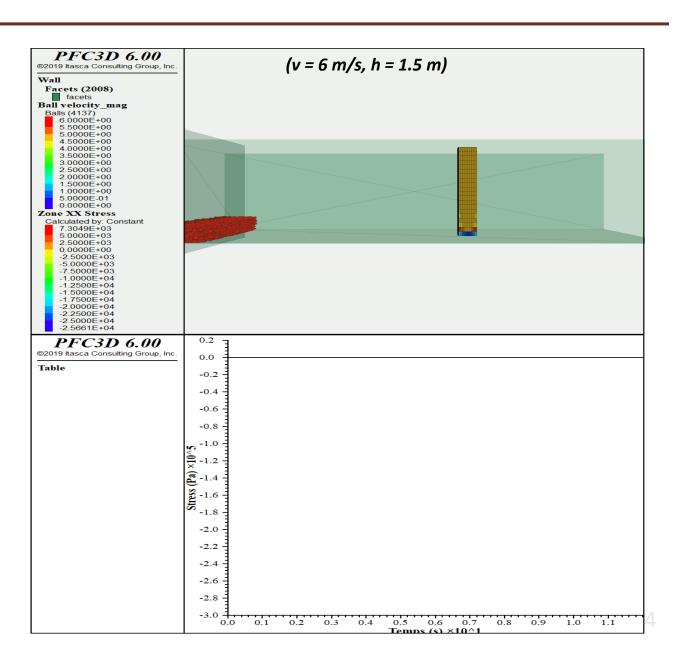
- Obtain a flow whose average velocity and fluid height is fixed at a certain distance from the obstacle.
- Calibration of the BC and the slope to obtain the desired characteristics of the flow.

Coupling steps:

- 1. Fluid calculation in Telemac3D to obtain the velocity vectors and the free surface height
- 2. Analysis and verification of results
- 3. Exporting the CFD file to PFC3D







4. Parametric study

• Code Telemac3D / influence of h, v and ρ s

Case	Flow height h	Nb Froude 0,79 0,55		
ref case	1.5 m	0,79		
Case 2	3 m	0,55		
Case 3	4 m	0,5		

Case	Blocks density ρs
ref case	2500 kg/ m^3
Case 2	$2700~{ m kg}/m^3$
Case 3	3000 kg/ m^3

Case	Velocity v Nd Froud		
ref case	3 m/s	0,79	
Case 2	4,5 m/s	1,18	
Case 3	6 m/s	1,56	

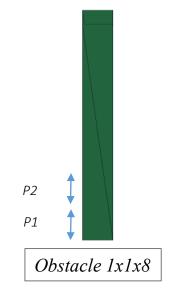
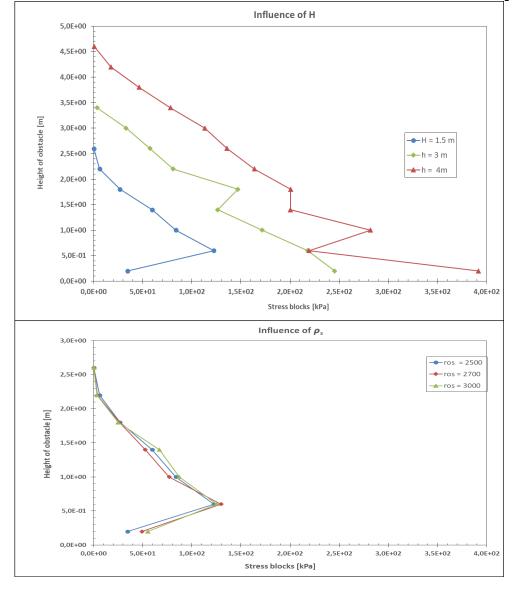


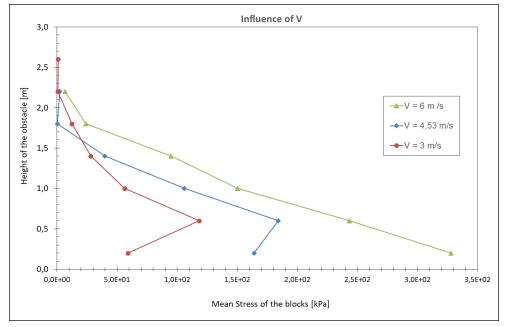
TABLE 3. Typical Values of Basic Physical Properties of Debris Flow Mixtures

Property and Unit	Symbol	Typical Values
Solid Grai	n Properties	
Mass density, kg/m ³	ρ_s	2500-3000
Mean diameter, m	δ	$10^{-5}-10$
Friction angle, deg	Φ_{g}	25-45
Restitution coefficient	ė	0.1-0.5
Pore Fluid	l Properties	
Mass density, kg/m ³	Pr	1000-1200
Viscosity, Pa s	μ	0.001-0.1
Mixture	Properties	
Solid volume fraction	υ,	0.4-0.8
Fluid volume fraction	v_f	0.2-0.6
Hydraulic permeability, m ²	k'	$10^{-13} - 10^{-9}$
Hydraulic conductivity, m/s	K	$10^{-7} - 10^{-2}$
Compressive stiffness, Pa	E	$10^3 - 10^5$
Friction angle, deg	ф	25-45

4. Parametric study

Influence of the flow parameters (h, v and hos) on the impact pressure of blocks on a structure





- ✓ When the flow height is higher, it is noticed that the pressure of the blocks on the obstacle is higher
- ✓ The impact of the blocks on the obstacle is much greater with a higher flow velocity
- ✓ Density influences the result of the impact but not strong enough as the two other parameters h and v

5. Comparison with empirical models

References

Zeng et al. 2015: Models of impact force of large boulders mixed in debris flow

Models	Description	Author
$F = 48,200 \cdot v^{1.2} \cdot R^2 \cdot g$	Derived from elastic collision theory modified by Mt. Yakedake's field investigation data. F impact force (N), ν velocity (m/s), R diameter of particles (m), g the acceleration of gravity (m/s 2)	Mizuyama (1979)
$F = 50,000 \cdot v^{1.2} \cdot R^2 \cdot g$	Derived from elastic collision theory modified by Myoukou field investigation data	Yamaguchi (1985)
$F=30,800 \cdot v^{1.2} \cdot R^2 \cdot g$	Derived from elastic collision theory modified by miniaturized test	Huang et al. (2007)
$F = c \left[\frac{M\nu^2(n+1)}{2c} \right]^{n/n+1}$	Derived from modified Hertz contact theory considering elastic to plastic deformation of barriers. M boulder mass (kg); coefficients c and n describe the character of barrier material	He (2010)
$F = \sqrt{Mv^2k}$	Equating the kinetic energy of the boulder with work expanded in bending deflection of beam. K is a stiffness factor of structure	Hungr et al. (1984)
The cantilever beam: $F = \sqrt{\frac{3EIV^2G'}{gL^3}}$ The simply supported beam:	Derived from material mechanics. E Young's modulus (N/m^2) , J inertia moment of transverse square to neutral axis (m^3) , G the weight of the boulder which is submerged in the debris flow (N) , L the length of the member	Zhang (1993)
$F=\sqrt{rac{48EJV^2G'}{gL^3}}$		

Hubl et al.2010 :
$$p_{\text{max}} = 5 \cdot \rho_{Mu} \cdot v^{0.8} \cdot (g \cdot h_{Mu})^{0.6}$$

Table 1. Debris flow properties esumated on near events based on Costa [0] and computed impact forces, empirical factors k and a, and the Froude-number

Torrent	h _{MU} [m]	$\rho_{Mu} [\mathrm{kg/m^3}]$	v [m/s]	p _{max} in MN/m ²	k**	a**	Fr**
Rio Reventado	8-12	1130-1980	2.9-10	0.7	4.67	18.67	0.50
Hunshui Gully	3-5	2000-2300	10-12	0.7	8.33	2.31	1.90
Bullock Greek	1.0	1950-2130	2.5-5.0	0.13	6.50	4.06	1.26
Pine Creek	0.1 - 1.5	1970-2030	10-31.1	0.3	21.43	0.38	7.56
Wrightwood Canyon (1969)	1.0	1620-2130	0.6-3.8	0.07	3.68	4.09	0.95
Wrightwood Canyon (1941)	1.2	2400	1.2-4.4	0.15	5.21	6.94	0.87
Lesser Almatinka	2.0-10.4	2000	4.3- 11.1	0.6	4.29	6.12	0.84
Nojiri River	2.3-2.4	1810-1950	12.7- 13.0	0.44	10.07	1.37	2.71

^{*} Mean values and based on the Hübl & Holzinger Formula

Hong et al. 2014: Statistics of the maximum impact pressure and total discharge from 1960 to 2000 in Jiangja Ravine, China

Parameter	Value	Value					
		ximum impact Total discharge Q_{Total} ssure P_{max} (kPa) (m ³)					
	Surge flow	Continuous flow	Surge flow	Continuous flow			
Mean	221	114	240,447	161,169			
Standard deviation (SD)	102	96	248,490	281,331			
Coefficient of variation	0.5	0.8	1.0	1.7			
Maximum	744	434	1,260,549	1,751,537			
Minimum	31	14	166	269			
Mean, SD of Lognormal distribution	5.3, 0.4	4.5, 0.7	12.0, 0.9	11.3, 1.2			
α, β for Weibull distribution*	2.3,	1.2, 120	1.0,	0.6,			
•	249.4		237,000	107,918			
α, β for Gamma distribution*	4.7, 46.8	1.4, 81.5	1.0,	0.3,			
-			256,803	491,083			

 $^{^*\}alpha$ = shape parameter; β = scale parameter.

Hu et al.2011: hydrodynamic models, p is the impact pressure

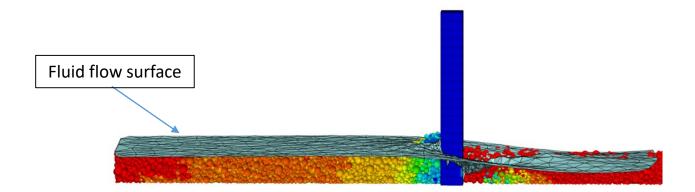
$$P=k\rho v$$
 or $P=k\rho v^2cos^2\theta$

$$p = c_m \left(0.5 \rho g h + \rho V^2 \right)$$

^{**} Mean values

5. Comparison with empirical models

Model analysis - Reference case /Telemac3D



✓ Numerical model: h fluide ≈ h blocs

✓ Analytical model (Hubl et al. 2010)

$$p_{\text{max}} = 5 \cdot \rho_{Mu} \cdot v^{0.8} \cdot (g \cdot h_{Mu})^{0.6}$$
 P max = 155 kPa

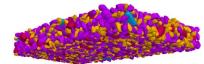
6. Conclusions and outlooks

Conclusions

- ✓ Make a flow model able to reproduce desired macro characteristics
- ✓ Use a simplified method to model the fluid with the aim of obtaining a fluid flow field around the obstacle
- ✓ Measuring the forces induced by the blocks on the structures
- ✓ Comparison of effort results with existing analytical models

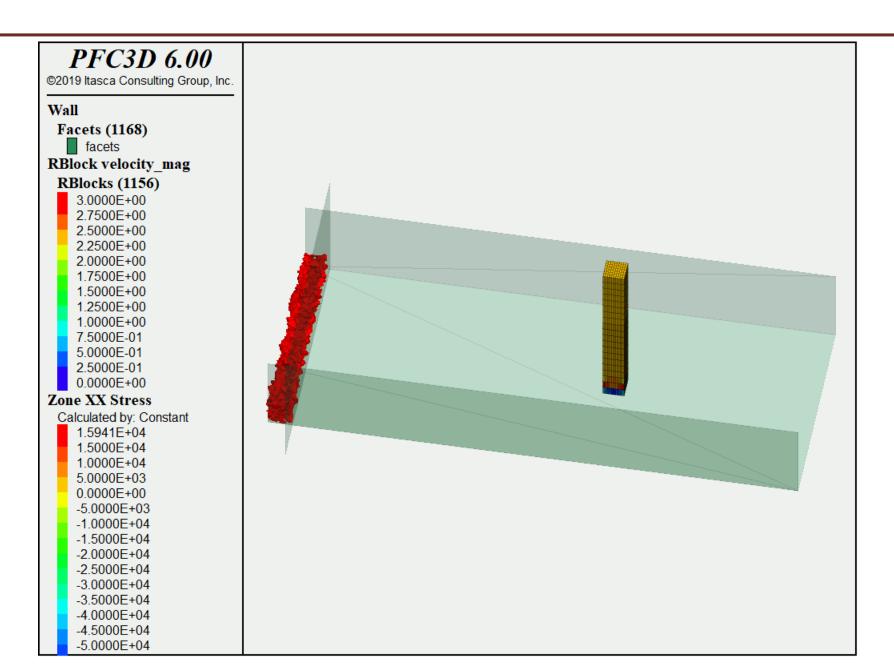
Outlooks

✓ Effect of other parameters remains to be identified e.g.: blocks size and shape, dynamic viscosity, orientation and position of obstacles...



- ✓ Use efforts generated by debris flows (blocks + fluid) to quantify the vulnerability of structures
- ✓ Analyze the damage from the flow intensity, the impact pressure on the structures by bibliography.

6. Conclusions and outlooks



Thank you for your attention



REFERENCES

Pallares, C., Fabre, D., Thouret, J. C., Bacconnet, C., Charca-Chura, J. A., Martelli, K., .. & Yanqui-Murillo, C. (2015). Geological and geotechnical characteristics of recent lahar deposits from El Misti volcano in the city area.

Thouret, J. C., Ettinger, S., Guitton, M., Santoni, O., Magill, C., Martelli, K., & Arguedas, A. (2014). Assessing physical vulnerability in large cities exposed to flash floods and debris flows: the case of Arequipa (Peru). Natural Hazards, 73(3), 1771-1815.

Hübl, Johannes et al. 2009. "Debris Flow Impact Estimation." *Eleventh international symposium on water management and hydraulic Engineering* (I): 137–48. http://wmhe.gf.ukim.edu.mk/Downloads/PapersTopic1/A56-Hubl-Suda-Proske-Kaitna-Scheidl.pdf.

Thouret, J. C., Enjolras, G., Martelli, K., Santoni, O., Luque, J. A., Nagata, M., ... & Macedo, L. (2013). Combining criteria for delineating lahar- and flash flood-prone hazard and risk zones for the city of Arequipa, Peru. Natural Hazards and EarthSystemSciences, 13(2), 339-360

Cundall, P. A., and O. D. L. Strack. "A Discrete Numerical Model for Granular Assemblies," Geotechnique, 29(1), 47-65 (1979).

Bugnion "Measurements of hillslope debris flow impact pressure on obstacles "(2011). Iverson, Richard M. 1997. "Of Debris." *Review of Geophysics* 35(97): 245–96.

Itasca, « Manuel d'utilisation du logiciel PFC 6.0 », 2018.

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