

NON-LINEAR STRANING OF FOUNDATION SOILS IN THE PROGRESSIVE FAILURE OF THE MOUNT POLLEY TSF EMBANKMENT

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Mine

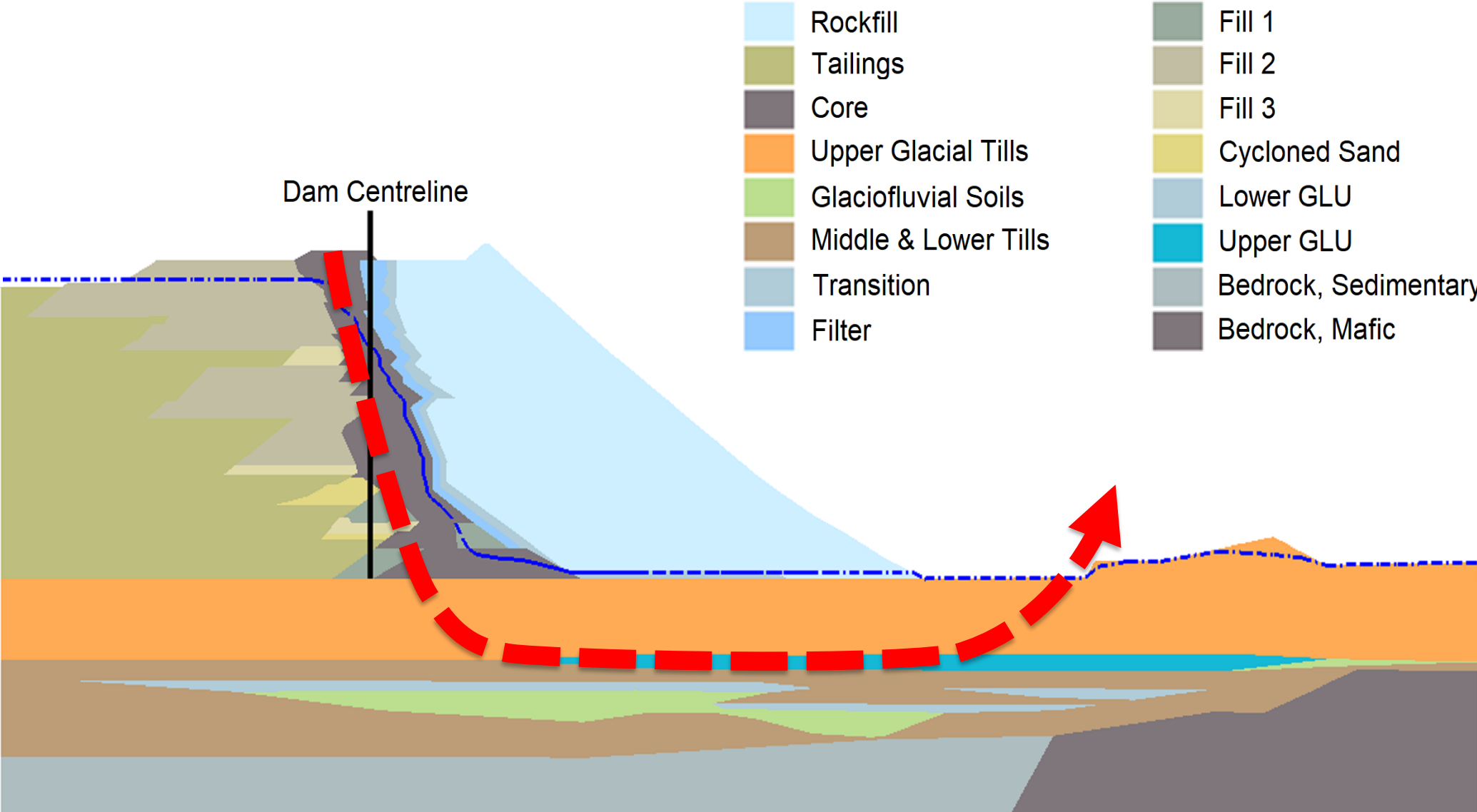
Bootjack Lake

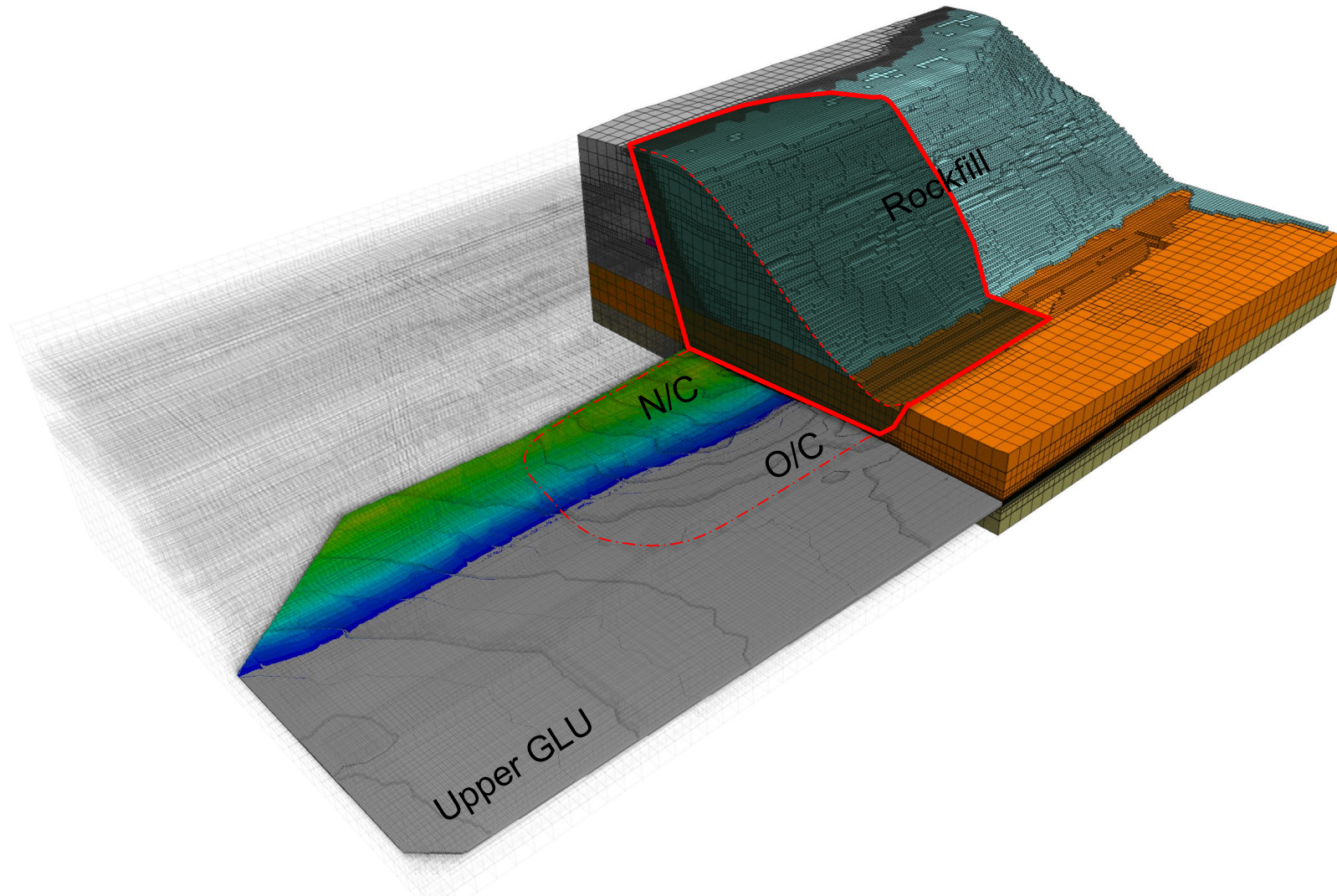
Polley Lake

Tailings Pond

Hazeltine Creek

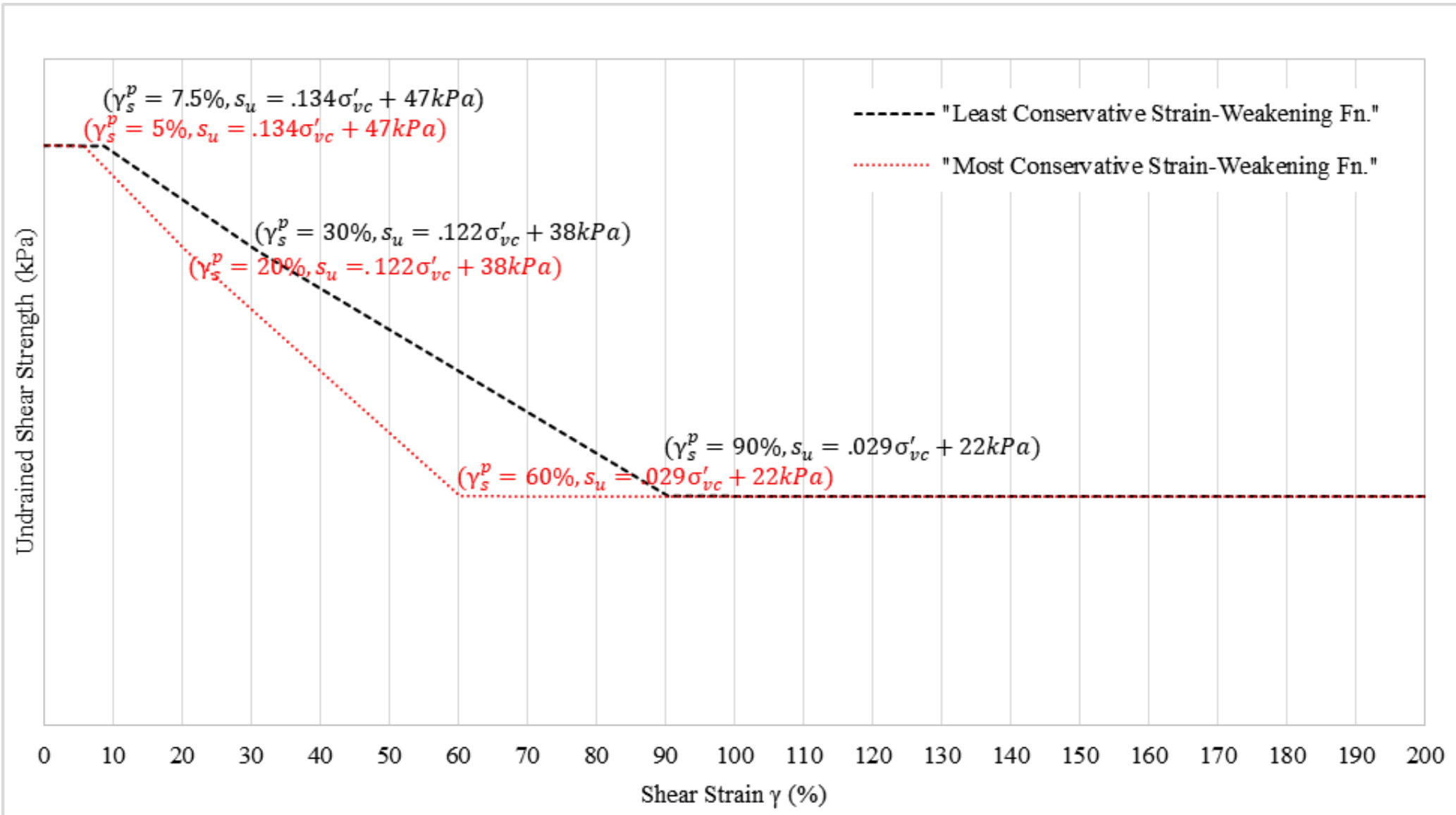
PRE-FAILURE PROFILE



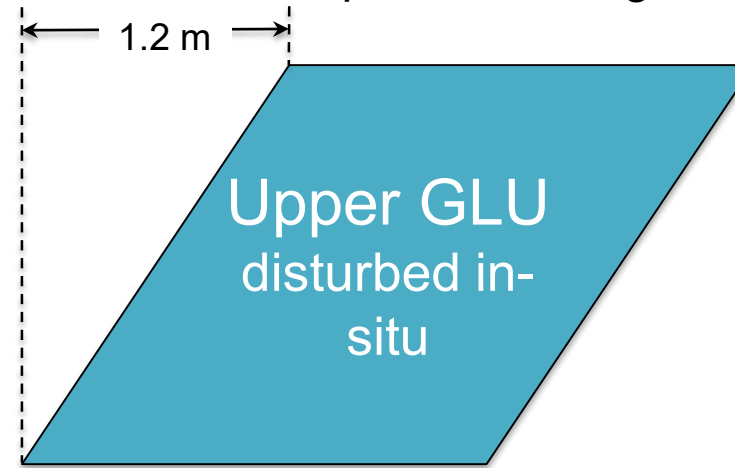
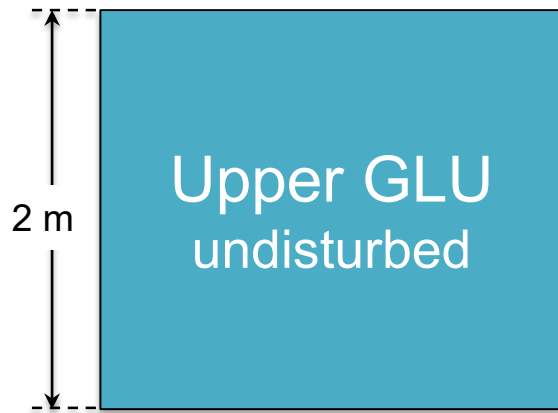


Static 3D analysis: $FOS = 1 @ s_{u,residual}$

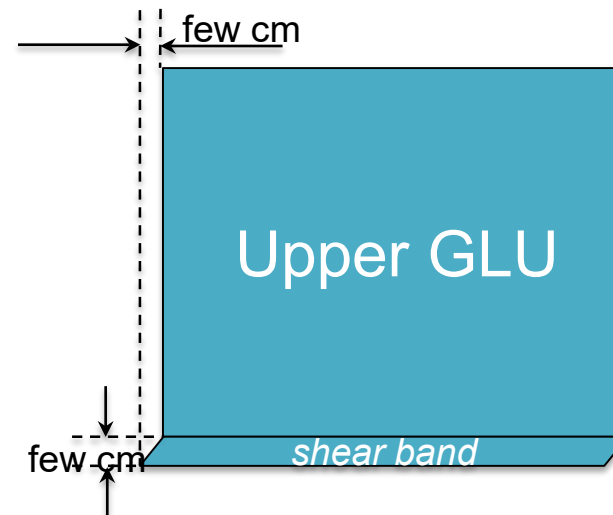
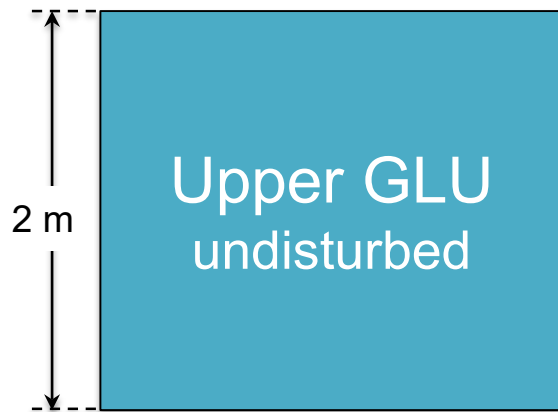
DSS-derived strain-weakening curves:



- From lab tests: >60% shear strains = full weakening
- In a 2m layer, this suggests shear displacements of 1.2 m before collapse if straining is uniform

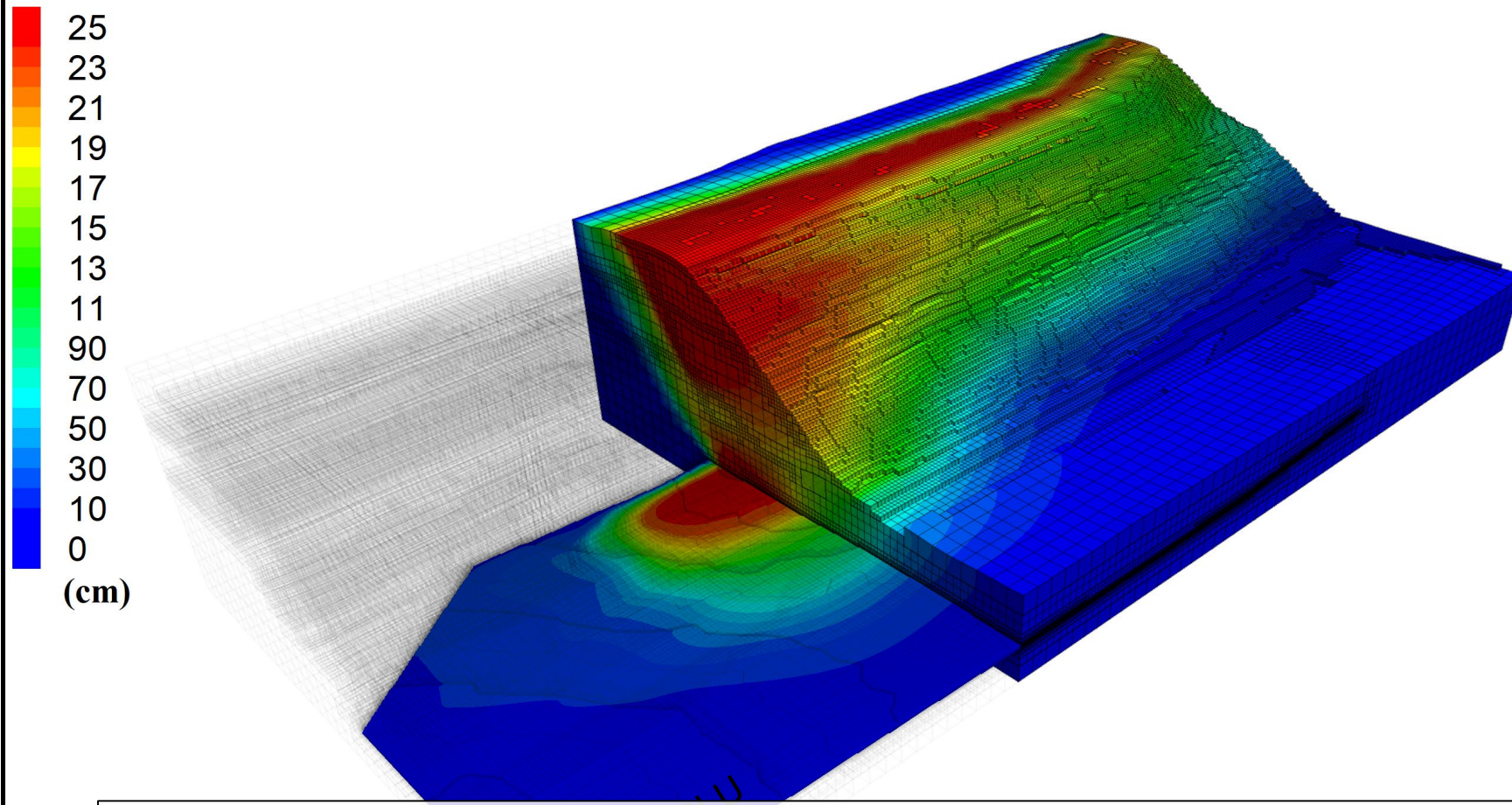


- From field observation: no such deformations happened pre-collapse

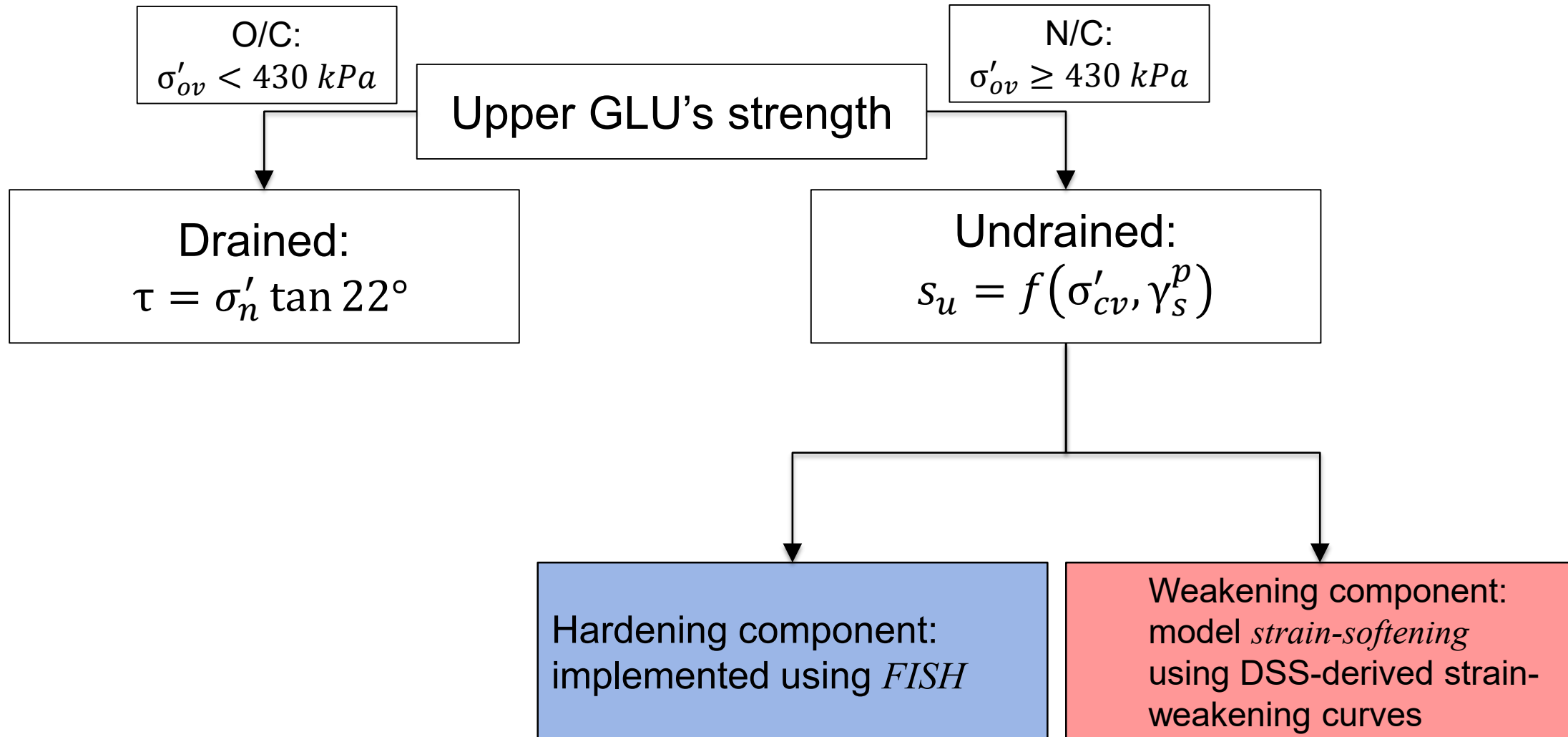




Incremental horizontal displacements during collapse



- $0 < \text{shear band} \leq 12\text{cm}$
- distinctly asynchronous shear strength mobilization

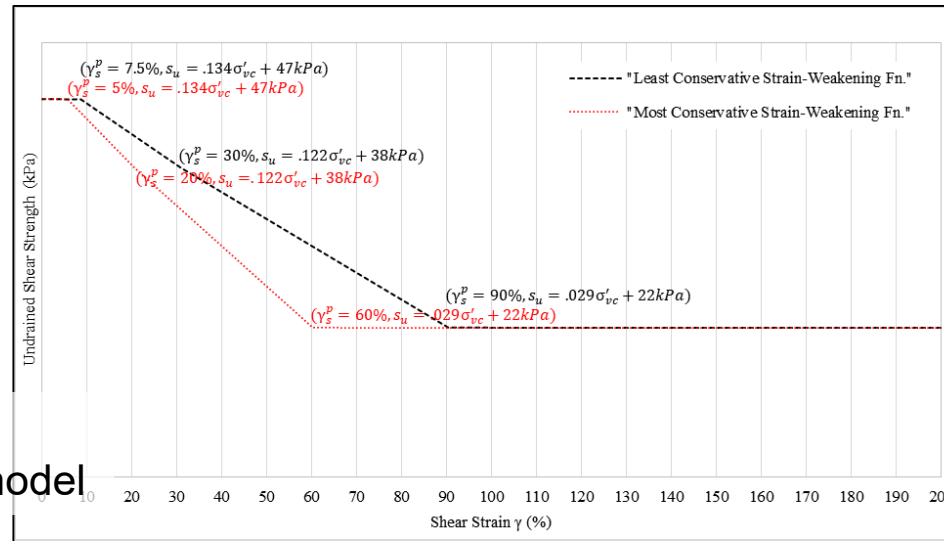
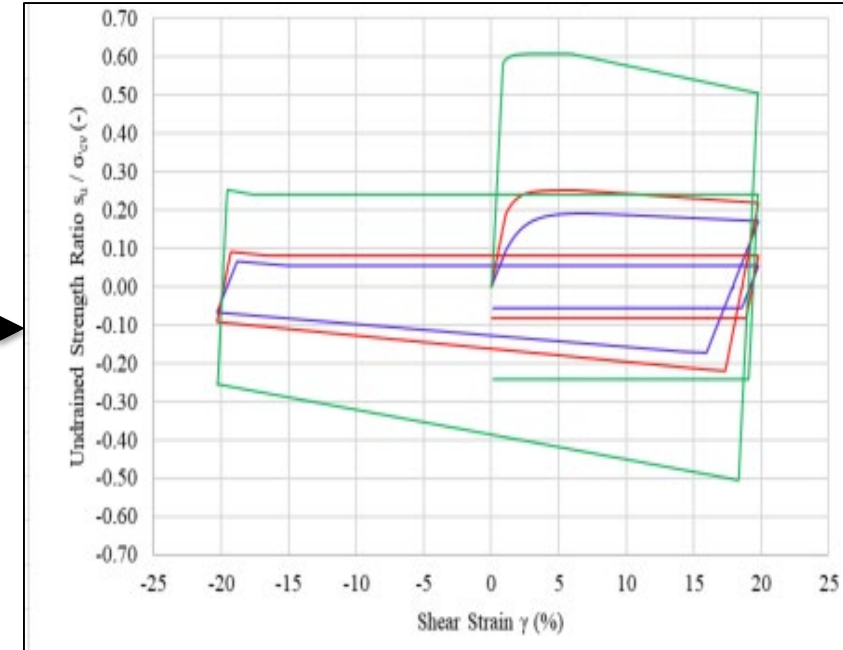


Actual DSS data (from KCB 2015, Fig. 5.23)

KCB's Direct Simple
Shear Testing Data

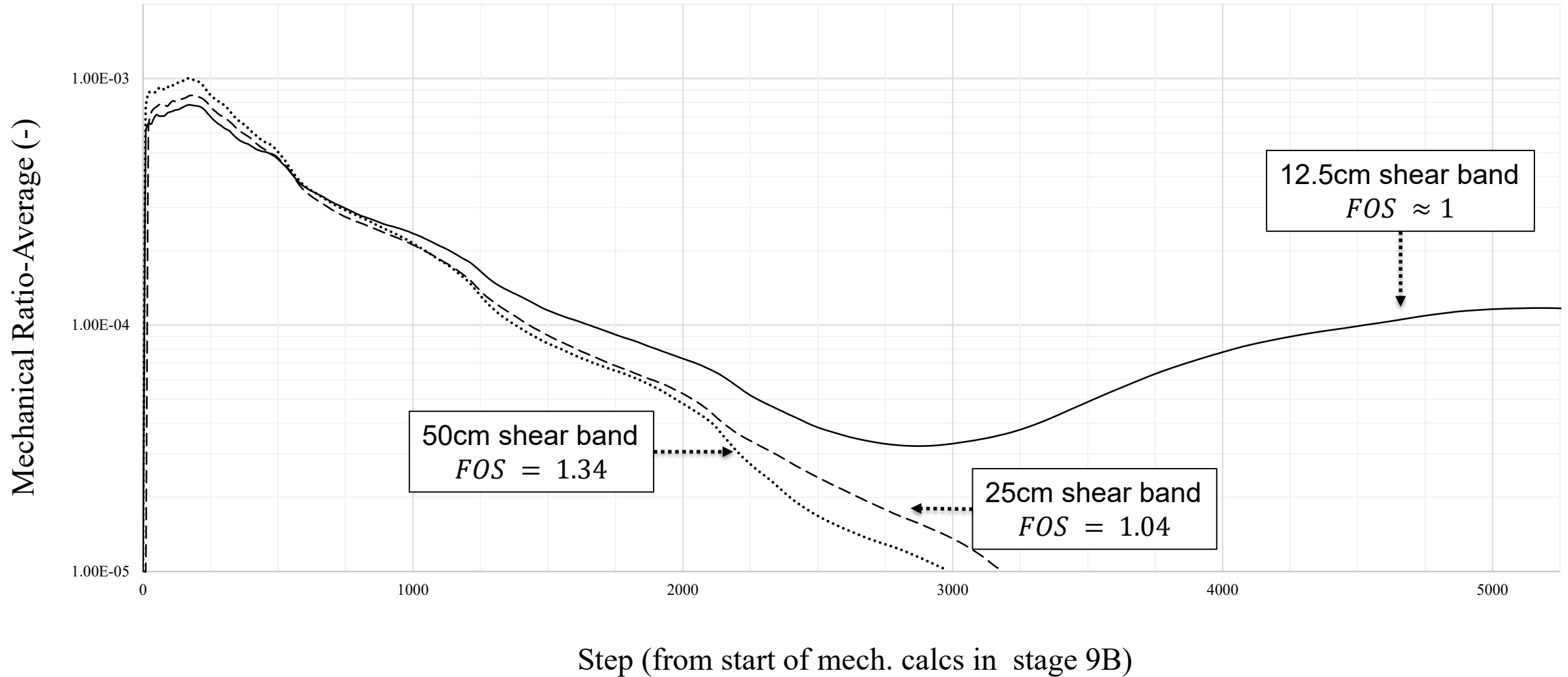
calibration

FLAC3D-simulated DSS data



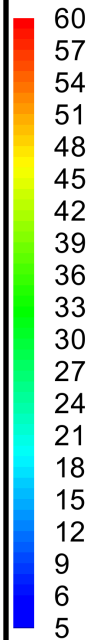
OUTPUT:
strain-weakening curves to be used in model

MODEL RESPONSE TO LOADING IN THE FINAL CONSTRUCTION STAGE (9B) using the most conservative interpretation of the strain-weakening curve in the Upper GLU

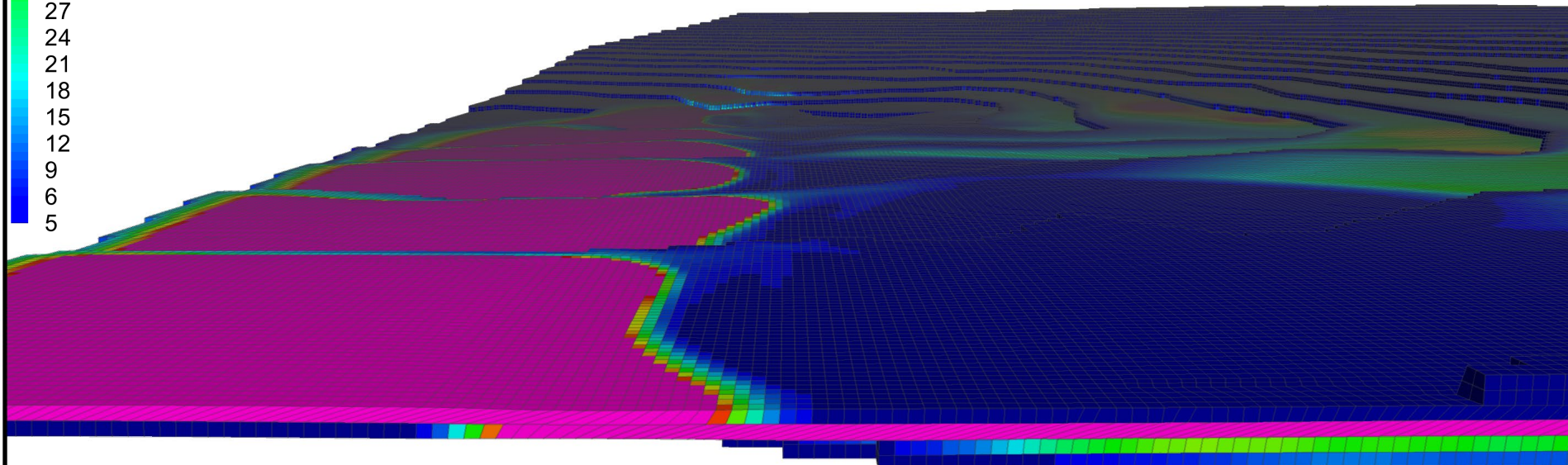


2014 - END OF COLLAPSE SIMULATION

plastic shear strains (%)



■ ≥ 60 (residual)
■ ≤ 5 (peak)

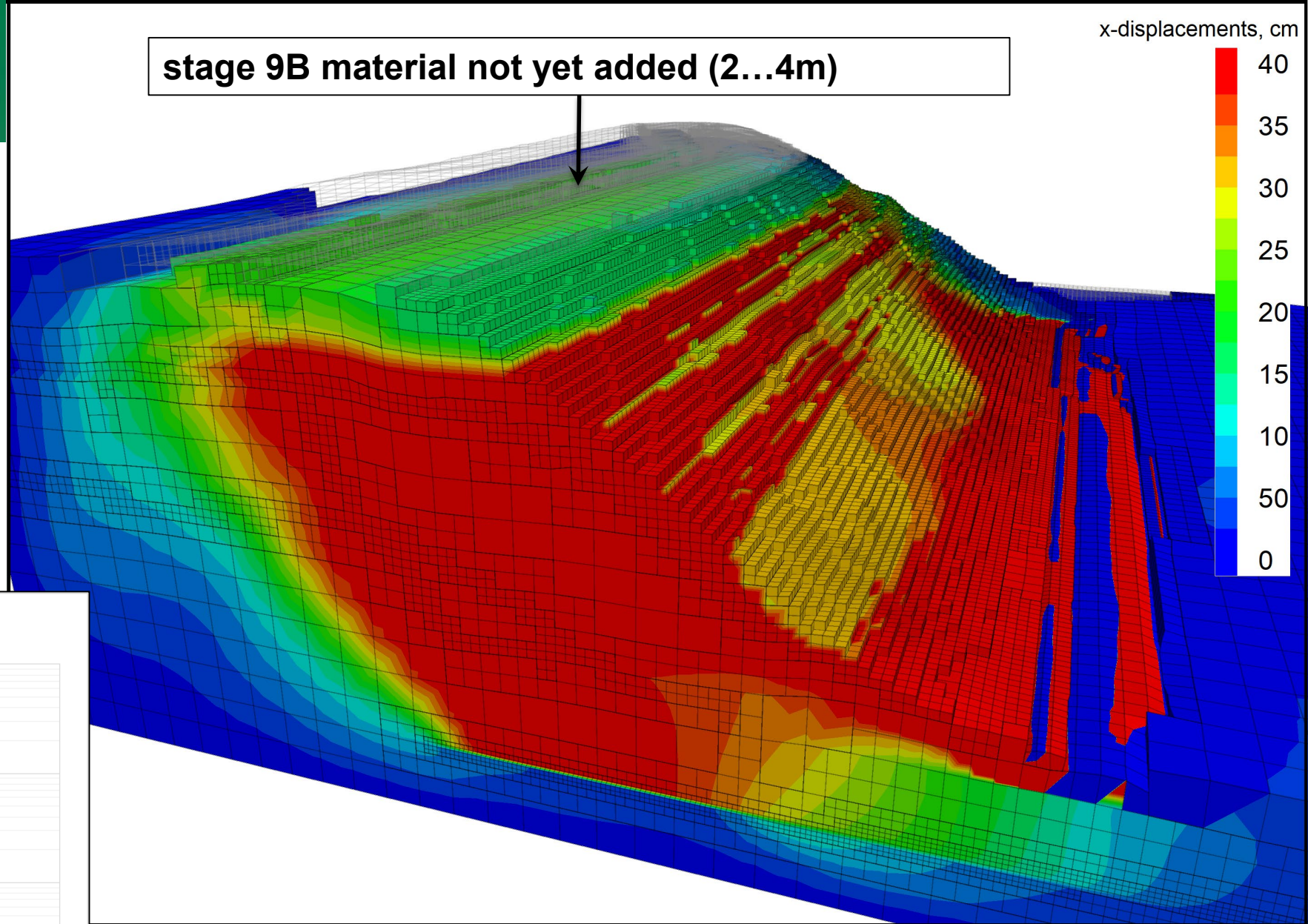


- The correct replication of failure in our finest mesh model can be a result of either:
 - (a) **Correct shear band thickness (12cm) combined with a correct choice of strain-weakening curve**
 - (b) **Excessive shear band thickness combined with an overly conservative choice of strain-weakening curve**
- Further mesh refinement computationally unfeasible
 - Conduct a **LOWER LIMIT STATE** analysis:
 - If mesh could be discretized indefinitely, zone height \rightarrow zero
 - Such zones, if prone to strain-weakening, would become fully weakened at zero plastic shear displacements
 - Apply “instant weakening” model to regular mesh to simulate “zero thickness” shear band

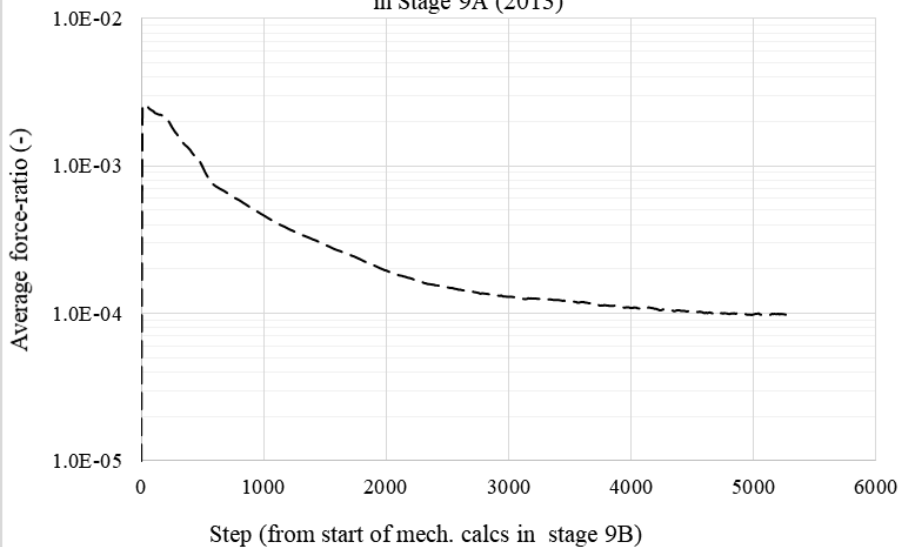
LOWER LIMIT STATE

Early collapse in stage 9A
(2013)

stage 9B material not yet added (2...4m)



LOWER LIMIT STATE ANALYSIS
Mechanical Ratio-Average vs. Calculation Step
in Stage 9A (2013)



From scale effects: shear band no thicker than 12.5cm

From analysis of lower limit state: : shear band > 0

$0 \text{ cm} < \text{SHEAR BAND} \leq 12.5 \text{ cm}$



ACKNOWLEDGMENTS

We are grateful to Itasca and its Educational Partnership for providing us, free of charge, with FLAC3D software used to complete the study of progressive failure at the Mount Polley TSF

A special thank you goes to **Augusto Lucarelli** for his mentorship through the more challenging modelling aspects of this problem

PHOTO CREDITS

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